

March 30, 2010 File No. 1-08-3220 Brampton Office

G. Merulla Inc. c/o Beacon Planning Services 3464 Semenyk Court, Unit 213 Mississauga, Ontario L5C 4P8

Attention: Mr. Dirk Blyleven

**RE: ADDENDUM-**

GEOTECHNICAL INVESTIGATION 2935 & 2955 MISSISSAUGA ROAD MISSISSAUGA, ONTARIO

Dear Mr. Blyleven:

This addendum letter summarizes the additional work/investigation carried out for the above noted property subsequent to the completion of our original Slope Stability and Streambank Erosion study in 2008.

#### **BACKGROUND**

Terraprobe Limited was retained by G. Merulla Inc. c/o Beacon Planning Services in 2008 to conduct a detailed slope stability and streambank erosion study for the subject property located in the southeast quadrant of the intersection of Mississauga Road and Dundas Street West, in the City of Mississauga, Ontario (Figure 1). The property consists of two adjoining land parcels (2935 - 2955 Mississauga Road). The property is currently vacant, however, includes remnants of previous development (abandoned swimming pool, concrete pad and a portion of the concrete foundation of a former dwelling) located on the south parcel (2935 Mississauga Road).

stoneycreek@terraprobe.ca

The field investigation for the original study (2008) was conducted on August 14, 2008, and consisted of drilling and sampling a total of four (4) exploratory boreholes extending to depths varying from about 7.4 m (Boreholes 2 and 4) to 9.2 m (Boreholes 1 and 3) below existing ground surface. A detailed slope stability and streambank erosion analysis was carried out utilizing site specific subsurface information obtained from these borehole and topographic survey information prepared by an Ontario Land Surveyor (Tarasick MacMillan Kubicki Limited, File No. 4871-08-T).

The location of the long-term stable slope crest was determinated and delineated based on the applicable stability and toe erosion setbacks in accordance with Credit Valley Conservation Policy Guidelines. The results of the investigation were presented in our report (Geotechnical Investigation, Slope Stability and Streambank Erosion Analysis, 2935 & 2955 Mississauga Road, Mississauga, Ontario, dated September 4, 2008). Figure 3 and Figures 5A and 5B of this report presented the location of the Long-term Stable Slope Crest in plan and sections. These figures delineated the location of the Long-term Stable Slope Crest where it was located beyond/behind the Physical Top of Bank (inland, towards the tableland), including for both east and south slopes. In other areas (including slope along north property boundary), it was concluded that the existing slope (Section 6, slope inclination 3 horz. to 1 vert.) is stable in the long-term, and the physical Top of Bank location in this area is considered to be the long-term stable slope crest location.

Subsequently, additional survey was carried out for the property by Tarasick MacMillan Kubicki (November 17, 2009) and the Topography Plan was updated. The updated plan included the Regional Flood Line (Elev. 98.0 m) staked at the south/southwest portion of the property (in areas where there is no significant and well defined slope present), as well as the surveyed Top of Bank for the sloping ground located at the north property boundary.

Terraprobe was contacted to conduct additional work/investigation for the property which included advancing additional shallow boreholes staked out by Tarasick MacMillan Kubicki along the surveyed 98.0m Elevation line located on the east/north side of the drainage ditch situated primarily within the southwest portion of the property, and a visual inspection of the site.

#### FIELD PROCEDURE

The number and locations of the boreholes were finalized by the client, as noted on Figure 3A. The boreholes were advanced to depths varying from about 1.2 to 2.1 m depth below grade. This supplementary subsurface soil investigation was conducted on March 17, 2010. The borehole ground surface elevations were provided by the surveyor. It should be noted that the borehole surface elevations are for the purpose of relating borehole soil stratigraphy and should not be used or relied on for other purposes.

The test holes were advanced by a specialist subcontractor with continuous soil sampling using a portable manual equipment. The in-situ penetration resistance testing was conducted manually by advancing a 51 mm diameter O. D. split spoon sampler with a 31.8 kg hammer, dropping a height of 762 mm. It should be noted that the weight of hammer utilized for advancing the split spoon sampler was half the weight of the normal drop hammer used in the Standard Penetration Test (ASTM D 1586) to measure the "N" values (blows/305mm of sampling spoon penetration), and therefore, the field "N" values were corrected accordingly.

Representative soil samples from the test holes were collected using the split spoon sampler during the performance of in-situ penetration tests. The field work (drilling, sampling, testing) was observed and recorded by a member of our engineering staff, who also transported the samples to our geotechnical testing laboratory.

The soil samples were visually examined, sealed into plastic jars, and transported to our laboratory where the samples were re-examined (tactile) in detail, and classified according to visual and index properties. Laboratory testing consisted of water content determination on all samples; and sieve and hydrometer analysis on selected soil samples (Borehole 1603, Sample 2; Borehole 1605, Sample 2; Borehole 1607, Sample 2; Borehole 1608, Sample 2; and Borehole 1611, Sample 1). Atterberg Limits tests were also conducted on selected soil samples (Borehole 1603, Sample 2; Borehole 1605, Sample 2; Borehole 1607, Sample 2; and Borehole 1611, Sample 1). The measured natural water content of the individual samples are plotted on the enclosed borehole logs at respective sampling depths, and the results of the sieve and hydrometer analysis as well as Atterberg Limits tests are appended.

Ground water levels were monitored in open boreholes upon completion of drilling and are recorded on the borehole logs.

#### SUBSURFACE CONDITIONS

The results of the individual boreholes are summarized below and recorded on the accompanying Borehole Logs. This summary is intended to correlate this data to assist in the interpretation of the subsurface conditions. Please refer to enclosed borehole logs for borehole and stratigraphic details.

It should be noted that the soil conditions are confirmed at the borehole locations only and may vary between and beyond the boreholes. The stratigraphic boundaries as shown on the logs represent an inferred transition between various strata, rather than a precise plane of geologic change.

#### **Topsoil**

A topsoil layer, varying in thickness from about 150 mm (Borehole 1608) to 200 mm (Borehole 1604) was encountered at the ground surface in Boreholes 1603, 1604, 1605, 1606, 1607 and 1608. A distinct topsoil layer was not encountered at the ground surface at the remaining boreholes (Boreholes 1609, 1610, 1611 and 1612) except for a presence of minor surficial organics/rootlets. However, presence of topsoil/organics (about 300 mm thick zone) was noted in these boreholes (Boreholes 1609, 1610, 1611 and 1612) at a depth of about 1 m below ground surface. The topsoil was dark brown to black in colour and predominantly consisted of a silt matrix. It must be noted that the topsoil thickness is estimated from the boreholes, and may vary between and beyond the boreholes.

#### **Earth Fill**

A layer of earth fill materials (about 1.0 m thick) was encountered at the ground surface in Boreholes 1609, 1610, 1611 and 1612. The earth fill materials predominantly consisted of clayey to sandy silt, silty sand, with trace to some gravel, and trace amounts of organics as well as rock/shale fragments. As noted above, the earth fill materials at these boreholes were underlain by topsoil/organic (about 300 mm thick) zone.

The Standard Penetration Test result ('N' Value) obtained from the earth fill generally varied from 3 to 11 blows per 300 mm of penetration, indicating typically a very loose to loose relative density (cohesionless soils) and firm consistency (cohesive soils). Measured moisture content of the earth fill samples varied from 12 to 21 percent by weight, indicating a typically moist condition.

#### **Native Soils**

Native soils were encountered beneath the surficial topsoil layer in Boreholes 1603, 1604, 1605, 1606, 1607 and 1608, and beneath the earth fill and embedded topsoil/organic layer at Boreholes 1609, 1610, 1611 and 1612. The native soils predominantly consisted of clayey to sandy silt, to sand and silt to silty sand till, with trace to some gravel to gravelly, and included sporadic presence of shale fragments in some boreholes. Weathered shale was encountered in Boreholes 1603, 1604, 1607 and 1612 beneath the native soil deposit at depths varying from about 1.2 to 2.0 m below grade.

The Standard Penetration Test results ('N' Values) obtained from the native soils generally varied from 5 to 16 blows per 300 mm of penetration, indicating a loose to compact relative density (cohesive soils) and firm to very stiff consistency (cohesive soils). Measured moisture contents of the native soil samples typically varied from 13 to 24 percent by weight, indicating a moist to very moist/locally wet condition.

#### **Ground Water**

Observations pertaining to the depth of water level and borehole caving were made in the open boreholes immediately after completion of drilling, and are noted on the enclosed borehole logs. A summary of these measurements is provided below.

Borehole No.	Test Hole Depth	Depth to Cave	Unstabilized Water Level Depth
1603	1.3 m BG	0.6 m BG	0.5 m BG
1604	1.2 m BG	1.1 m BG	0.9 m BG
1605	1.2 m BG	minor caving	1.0 m BG
1606	1.2 m BG	open	1.0 m BG
1607	1.8 m BG	1.2 m BG	1.1 m BG
1608	1.2 m BG	open	dry
1609	1.8 m BG	open	dry
1610	1.8 m BG	open	dry
1611	1.8 m BG	open	dry
1612	2.1 m BG	1.5 m BG	dry

BG = Below Grade

It should be noted that the ground water levels indicated above may fluctuate seasonally depending on the amount of precipitation and surface runoff.

#### SITE INSPECTION AND REVIEW

The site was visited on March 25, 2010 to conduct a visual inspection to assess general physical site conditions. Our original site investigation and study was carried out in 2008, therefore, the visual site inspection was carried out to establish if there has been any significant changes (regrading) of the site areas which may have an implication of our previous findings and recommendations.

As noted before, an updated topographic plan was provided (prepared by Tarasick MacMillan Kubicki) which included location of the Regional Flood Line (Elev. 98.0 m) staked at the south/southwest portion of the property (in areas where there is no significant and well defined slope present), as well as the surveyed Top of Bank for the sloping ground located at the north property boundary.

A review of the updated plan and site inspection indicated that there has been no significant changes to the general physical condition of the site particularly with respect to site grades. There was no conspicuous indication of any further cut/fill of the site or significant grade alterations which may have implications on our previous (2008) findings and recommendations.

Similar to the conditions noted during our previous site investigation in 2008, the majority of the grade alteration ('relatively recent' cut and fill) was limited to the area located approximately within the northerly two-third portion of the site and was contained within the silt fenced area. The southerly one-third portion of the site (located approximately to the south of the northerly line of the remnant of the in-ground concrete pool and circumferenced by the silt fence) does not appear to have experienced any 'recent' grade alterations (cut/fill), which is also evident from the presence of patchy surficial vegetation in this area which is not present in the northerly two-third ('recent cut/fill') portion of the site (Photos A, B and C).

As previously indicated, the updated plan includes the location of the Regional Flood Line (Elev. 98.0 m) staked at the south/southwest portion of the property (in areas where there is no significant and well defined slope present). Our slope and erosion study (2008) delineated the location of the Long-term Stable Slope Crest (LTSSC) where it was located beyond/behind the Physical Top of Bank (inland, towards the tableland), including for both east and south slopes as noted in Figure 3 of our original report. There is currently, a generally wooded area, located to the south and between the southerly end of our LTSSC line (close to Borehole 4) established for the east slope (along Credit River) and the easterly extremity of the LTSSC line drawn for the southerly ridge (Sections 5 and 6), which does not include a well defined slope. Although there is no well defined slope located in this area and the ground inclination is relatively very gentle (about 4 to 6 horz. to 1 vert., or flatter), we recommend that the LTSSC line for this area be established by joining the previously established southerly end of the LTSSC line for the easterly slope, and the easterly extremity of the LTSSC line drawn for the southerly ridge area. Although, a very conservative approach, the LTSSC line obtained in this manner, for this areas, will ensure that it is located well above the Regional Flood Line elevation as well as it is located outside of any slope/valley feature(s) which may be present in this area. The location of the Long-Term Stable Slope Crest for all site slopes is presented on Figure 3A.

We trust the foregoing information is sufficient for your present requirements. If you have any questions, or if we can be of further assistance, please do not hesitate to contact us.

Yours truly,

#### Terraprobe Limited

B. Singh, M.A Associate

Michael Tanos, P. Eng.

Principal

encl. Abore vations, Terminology and General Information

Borehole Logs

Sieve and Hydrometer Analysis Atterberg Limits Test Results

Figure 1

Site Location Plan

Figure 2

Aerial Photograph (2006)

Figure 3A

Topographic Plan (Revised March 2010)

**PHOTOGRAPHS** 

# **ENCLOSURES**



TERRAPROBE INC.



# ABBREVIATIONS, TERMINOLOGY, GENERAL INFORMATION

#### **BOREHOLE LOGS**

SAMP	LING METHOD	PENETRATION RESISTANCE
SS ST AS WS RC	split spoon Shelby tube auger sample wash sample rock core	<b>Standard Penetration Test</b> (SPT) resistance ('N' values) is defined as the number of blows by a hammer weighing 63.6 kg (140 lb.) falling freely for a distance of 0.76 m (30 in.) required to advance a standard 50 mm (2 in.) diameter split spoon sampler for a distance of 0.3 m (12 in.).
WH PH	weight of hammer pressure, hydraulic	<b>Dynamic Cone Test</b> (DCT) resistance is defined as the number of blows by a hammer weighing 63.6 kg (140 lb.) falling freely for a distance of 0.76 m (30 in.) required to advance a conical steel point of 50 mm (2 in.) diameter and with 60° sides on 'A' size drill rods for a distance of 0.3 m (12 in.).

SOIL DESCRIPTION - COHE	SIONLESS SOILS	SOIL DESCRI	PTION - COHESIVE	SOILS
Relative Density very loose	'N' value	Consistency	Undrained Shear Strength, kPa	'N' value
loose compact dense very dense	4 - 10 10 - 30 30 - 50 > 50	very soft soft firm stiff very stiff hard	< 12 12 - 25 25 - 50 50 - 100 100 - 200 > 200	< 2 2 - 4 4 - 8 8 - 15 15 - 30 > 30
SOIL COMPOSITION		TESTS, SYMB	OLS	
'trace' (e.g. trace silt) 'some' (e.g. some gravel) adjective (e.g. sandy) 'and' (e.g. sand and gravel)	% by weight < 10 10 - 20 20 - 35 35 - 50	$\begin{array}{ccc} w,w_c & \text{water } \alpha \\ w_l & \text{liquid } l \\ w_p & \text{plastic} \\ l_p & \text{plastic} \\ k & \text{coeffic} \\ \gamma & \text{soil un} \\ \varphi' & \text{angle } \alpha \\ c' & \text{cohesi} \end{array}$		meter analysis

#### **GENERAL INFORMATION, LIMITATIONS**

The conclusions and recommendations provided in this report are based on the factual information obtained from the boreholes and/or test pits. Subsurface conditions between the test holes may vary.

The engineering interpretation and report recommendations are given only for the specific project detailed within, and only for the original client. Any third party decision, reliance, or use of this report is the sole and exclusive responsibility of such third party. The number and siting of boreholes and/or test pits may not be sufficient to determine all factors required for different purposes.

It is recommended Terraprobe be retained to review the project final design and to provide construction inspection and testing.

# BOREHOLE LOGS



# **LOG OF BOREHOLE 1603**

PROJECT: 2935 & 2955 Mississauga Roa	DATE:	March	17, 2010		
LOCATION: Mississauga, Ontario	EQUIPM	MENT: Manua	I SPT		
CLIENT: Mr. Merulla	ELEVA	TION DATUM:	Geodetic	FILE:	1-08-322

	SOIL PROFILE			SAMP	LES	Щ	PENE RESIS	TRATIO	N PLOT	_			NAT	UDAL		0 ~	STANDOIGE
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1.2 96.8 1.3	(Georgian Bay Formation)		3	SS	50/5cm								0				
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Borehole was caving at 0.6m (Elev. 97.5m) and unstabilized water level at 0.5m (Elev. 97.6m) upon completion of drilling.



CLIENT: Mr. Merulla

## **LOG OF BOREHOLE 1604**

FILE: 1-08-3220

ELEVATION DATUM: Geodetic

PROJECT: _	2935 & 2955 Mississauga Road	DATE:	March 17, 2010
OCATION:	Mississauga, Ontario	EQUIPMENT:	Manual SPT

Γ	SOIL PROFILE	,		SAMP	l E¢	111	PENE	TRATIC	N				I					1-00-0220
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97.9		17.31																
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	CLAYEY TO SANDY SILT		1															
	gravelly, trace shale fragments, stiff, greyish brown, very moist to wet		_		٠,													互
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NOTES:

Borehole was caving at 1.1m (Elev. 97.0m) and unstabilized water level at 0.9m (Elev. 97.2m) upon completion of drilling.

## **LOG OF BOREHOLE 1605**

 PROJECT:
 2935 & 2955 Mississauga Road
 DATE:
 March 17, 2010

 LOCATION:
 Mississauga, Ontario
 EQUIPMENT:
 Manual SPT

CLIENT: Mr. Merulla ELEVATION DATUM: Geodetic FILE: 1-08-3220

SOIL PROPRIE  SOIL PROPRIE  SOIL PROPRIE  DESCRIPTION  SOIL PROPRIE  SOIL PROPRIE  DESCRIPTION  SOIL PROPRIE  SOIL PRO		SOIL PROFILE			SAMP	LES	Щ	PENE	TRATIC	ON E PLOT					NAT	I IRAI		υ~	STANDPIPE
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0.2 trace organic/rootlets in upper ± 0.2m  soft  CLAYEY TO SANDY SILT some gravel, firm, brown, very moist to wet  2 SS 5  GF, SA.SI.CL 18.30.34.18	0.0		74.7				98												
CLAYEY TO SANDY SILT some gravel, firm, brown, very moist to wet 2 SS 5 GR.SA.SI.CL 18 30.34.18 O 1—1  96.9 (TILL)		soft			ss	2									0	***************************************			
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	_	End of Borehole																	

NOTES:

Borehole was caving at 1.2m (Elev. 96.9m) and unstabilized water level at 1.0m (Elev. 97.1m) upon completion of drilling.



## **LOG OF BOREHOLE 1606**

PROJECT: 2935 & 2955 Mississauga Road DATE: \_\_ March 17, 2010 LOCATION: Mississauga, Ontario EQUIPMENT: Manual SPT

CLIENT: Mr. Merulla ELEVATION DATUM: Geodetic \_ FILE: <u>1-08-3220</u>

SOIL PROPILE		SOIL PROFILE SAMPLES				LES	ij	PENE	TRATIC	N E PLOT	>				NAT	URA		U n	STANDPIPE
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trace to some gravel, firm, brown, very moist to wet  2 SS 5  (TILL)		trace organic/rootlets in upper ± 0.2m		1	SS	3										0			
96.9		trace to some gravel, firm, brown, very moist to wet		2	SS	5	07									o			Ā
1.2 End of Borehole		3	MM				97												
NOTES:																			

Borehole was open and unstabilized water level at 1.0m (Elev. 97.1m) upon completion of drilling.

## **LOG OF BOREHOLE 1607**

PROJECT: 2935 & 2955 Mississauga Road DATE: \_ March 17, 2010 LOCATION: Mississauga, Ontario EQUIPMENT: Manual SPT

<b>-</b> -	CLIENT: Mr. Merulla							ELEV	ATIO	_	TUM:			ic			FILE:	1-08-3220
	SOIL PROFILE			SAMP	LES	SCALE	PENE RESIS	TRATIO	ON E PLOT	2	-		PLAST	IC NAT	URAL	LIQUID	일 또	STANDPIPE
ELEV DEPTH 98.0		STRAT PLOT	NUMBER	ТҮРЕ	"N" VALUES	ELEVATION	SHEA O U	AR STI NCONE OCKET	RENG INED PEN.	TH kP + ×	a FIELD LAB V	_	W <sub>P</sub> WA¹		W O ONTEN	LIMIT W L	G ORGANIC 3 VAPOUR	INSTALLATION OR REMARKS
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NOTES:

Borehole was caving at 1.2m (Elev. 96.8m) and unstabilized water level at 1.1m (Elev. 96.9m) upon completion of drilling.

## **LOG OF BOREHOLE 1608**

PROJECT: 2935 & 2955 Mississauga Road DATE: March 17, 2010

LOCATION: Mississauga, Ontario EQUIPMENT: Manual SPT

CLIENT: Mr. Merulla ELEVATION DATUM: Geodetic FILE: 1-08-322

	CLIENT: Mr. Merulla					TAPILUS .	_ [	ELEV	ATIO		ΓUM:		eodet	ic	****		FILE:	1-08-3220
4	SOIL PROFILE			SAMP	LES	J.E	PENE	TRATIC	N PLOT	_			DI ACT	, NAT	URAL	LIGUED	۷ د	STANDPIPE
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T THE THE THE THE THE THE THE THE THE TH	End of Borehole		To the transfer of the transfe															

NOTES:
Borehole was open and dry upon completion of drilling.



CLIENT: Mr. Merulla

# **LOG OF BOREHOLE 1609**

\_\_ FILE: <u>1-08-3220</u>

ELEVATION DATUM: Geodetic

PROJECT: _	2935 & 2955 Mississauga Road	DATE:	March 17, 2010
LOCATION:	Mississauga, Ontario	EQUIPMENT:	Manual SPT

	\$OIL PROFILE			SAMP	LES	J.	PENE	TRATIC	N PLOT	_				NAT	URAL		0 ~	STANDPIPE
ELEV DEPTH	DESCRIPTION  Ground Surface	STRAT PLOT	NUMBER	TYPE	"N" VALUES	ELEVATION SCALE	SHEA O UI	R STE NCONF	0 6 RENGT INED	0 8 H kPa + ×	FIELD LAB V	VANE	WP WAT	TER CO	W O ONTEN	LIQUID LIMIT VL T(%)	G ORGANIC S VAPOUR	INSTALLATION OR REMARKS
0.0	Surficial organic/rootlets presence FILL - Sandy Silt, some gravel, trace clay, with rock/shale fragments, topsoil/organic inclusions, loose, brown/grey, moist		1	SS	4	98								0				
97.1	TOPSOIL		2	SS	4	97								0	<b></b>			
96.8 1.3 96.3	CLAYEY TO SANDY SILT trace to some gravel, firm, brown, moist to very moist		3	ss	5										0			
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NOTES:

Borehole was open and dry upon completion of drilling.

Sheet 1 of 1



# **LOG OF BOREHOLE 1610**

PROJECT: _	2935 & 2955 Mississauga Road	DATE:	March 1	17, 2010		
LOCATION:	Mississauga, Ontario	EQUIPMENT:	Manual	SPT		
CLIENT:	Mr. Merulla	ELEVATION DA	TUM:	Geodetic	FILE:	1-08-3220

	SOIL PROFILE			SAMP	LES	щ	PENE	TRATIC	N PLOT									
ELEV DEPTH 98.0	DESCRIPTION	STRAT PLOT	NUMBER	түрЕ	"N" VALUES	ELEVATION SCALE	SHEA O UI	R STE NCONF	0 6 RENGT INED PEN.	0 8 H kPa + ×	80 10	VANE ANE	w <sub>P</sub> ⊢ WAT	ER CC	NTEN	LIQUID LIMIT V.L T (%)	G ORGANIC 3 VAPOUR	STANDPIPE INSTALLATION OR REMARKS
0.0	Surficial organic/rootlets presence FILL - Clayey Silt, some sand to sandy, some gravel, with rock/shale fragments, topsoil/organic inclusions, firm, brown, moist		1	ss	6	98							erit.	o				
97.1 0.9 96.8 1.2	TOPSOIL decayed wood/rootlet inclusions, dark brown to black		2	SS	4	97									0			
96.2 1.8	trace to some gravel, firm, brown, moist to very moist (TILL)		3	SS	5										0			
	TES:																	

Borehole was open and dry upon completion of drilling.

Sheet 1 of 1

## **LOG OF BOREHOLE 1611**

PROJECT: _	2935 & 2955 Mississauga Road	DATE:	March 17, 2010
LOCATION:	Mississauga, Ontario	EQUIPMENT:	Manual SPT

	CLIENT: Mr. Meruila											G	eodet	ic			FILE:	1-08-3220
	SOIL PROFILE	<u> </u>		SAMP	Γ	CALE	RESIS	TRATIO	E PLOT			00	PLAST LIMIT	TIC NAT	URAL STURE	LIQUID LIMIT	INIC	STANDPIPE INSTALLATION
ELEV DEPTH 98.1	DESCRIPTION  Ground Surface	STRAT PLOT	NUMBER	TYPE	"N" VALUES	ELEVATION SCALE	SHEA O U	AR STI NCONF OCKET	RENG INED PEN.	TH kPa + ×	FIELD LAB V	VANE ANE 00	W <sub>P</sub>	TER CO		₩L	G ORGANIC 3 VAPOUR	OR REMARKS
0.0	Surficial organic/rootlets presence FILL - Sandy Silt to Silty Sand, some gravel to gravelly, some clay, trace rock/shale fragments, topsoil/organic inclusions, loose, brown/grey, moist		1	ss	4	98	GF	3.SA.SI				Service Control of the	0 1	1	Proposition and the second			
97.1 1.0 96.8	decayed wood/rootlet inclusions,		2	ss	7	97			- Appropriate Action						0			
96.3	CLAYEY TO SANDY SILT trace to some gravel, firm, brown, moist (TILL)		3	ss	7									0				
1.8	End of Borehole																	

Borehole was open and dry upon completion of drilling.



## **LOG OF BOREHOLE 1612**

 PROJECT:
 2935 & 2955 Mississauga Road
 DATE:
 March 17, 2010

 LOCATION:
 Mississauga, Ontario
 EQUIPMENT:
 Manual SPT

CLIENT: Mr. Merulla ELEVATION DATUM: Geodetic FiLE: 1-08-3220

PLASTIC MOISTURE LIQUID  STORY	SOIL PROFILE			SAMP	LES	щ	PENE	TRATIC	N		····							
Surficial organic foctories presence FILL - Clayey to Sandy Silt, some gravel, with rock/shale fragments, topsoil/organic inclusions, obstruction at 0.8m, very loose, brown, moist  compact  2 SS 11  96.9  1.1 TOPSOIL decayed wood/rootlet inclusions, dark brown to black  1.4 CLAYEY TO SANDY SILT trace to some gravel, stiff, brown, moist to very moist  (TILL)  4 SS 12 96  0 O	98.0	DESCRIPTION E	SINAI PLOI				ELEVATION SCAL	SHEA O UI	R STF NCONF	0 6 RENGT INED PEN.	0 8 H kPa + ×	FIELD V	VANE NE	W <sub>P</sub> WAT	ER CC	NTEN	₩ <sub>L</sub> Γ(%)	STANDPIPE INSTALLATION OR REMARKS
compact  2 SS 11  96.9  1.1 TOPSOIL decayed wood/rootlet inclusions, 96.5 dark brown to black  1.4 CLAYEY TO SANDY SILT trace to some gravel, stiff, brown, moist to very moist  (TILL)  4 SS 12 96  2 SS 11  97  4 SS 12  96  0 0	0.0	FILL - Clayey to Sandy Silt, some gravel, with rock/shale fragments, topsoil/organic inclusions, obstruction		1	ss	3									0			
decayed wood/rootlet inclusions, dark brown to black  1.4 CLAYEY TO SANDY SILT trace to some gravel, stiff, brown, moist to very moist  (TILL)  2.1 WEATHERED SHALE (Georgian Bay Formation)		 compact		2	SS	11	97				······································					<b>&gt;</b>		
99.9 WEATHERED SHALE 96.9 (Georgian Bay Formation)	96.5	decayed wood/rootlet inclusions, dark brown to black  CLAYEY TO SANDY SILT trace to some gravel,	; ; ; ; ; ;	3	SS	8									0	0		
95.9 (Georgian Bay Formation)				4	SS	12	96	_\										
NOTES:	95.9	(Georgian Bay Formation)  End of Borehole																

Borehole was caving at 1.5m (Elev. 96.5m) and dry upon completion of drilling.

# SIEVE AND HYDROMETER ANALYSIS





PROJECT: 2935 & 2955 Mississauga Road

LOCATION: Mississauga, Ontario

CLIENT: Mr. Merulla

BOREHOLE NUMBER: 1603 SAMPLE NUMBER: 2

SAMPLE DEPTH: 0.6 - 1.2 m

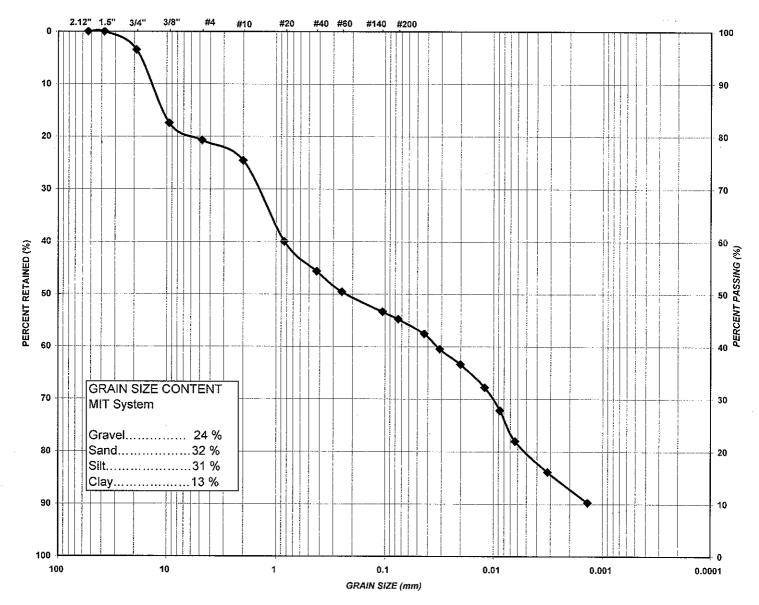
SAMPLE DESCRIPTION: SAND AND SILT, gravelly, some clay (TILL)

FILE NO.: **1-08-3220** LAB NO.: **1049A** 

SAMPLE DATE: March 17, 2010

SAMPLED BY: AW / JS

#### **GRAIN SIZE DISTRIBUTION**



MIT SYSTEM		GRAVEL			SAND	FINE	SILT	CLAY
UNIFIED	COARSE	FINE	COARSE	MEDIUM		FINE		
SYSTEM	GR.	AVEL		SANE	5		SILT AND	CLAY



PROJECT: 2935 & 2955 Mississauga Road

LOCATION: Mississauga, Ontario

CLIENT: Mr. Merulla

BOREHOLE NUMBER: 1605 SAMPLE NUMBER: 2

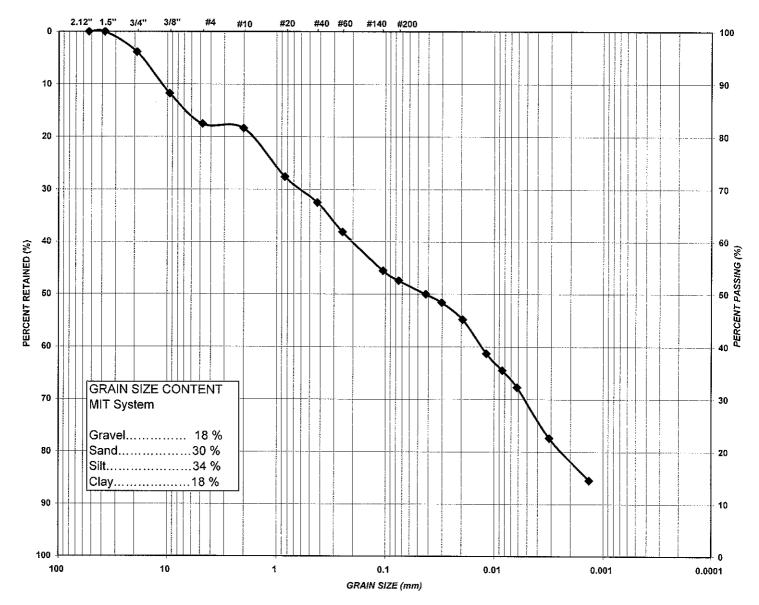
SAMPLE DEPTH: 0.6 - 1.2 m

SAMPLE DESCRIPTION: SANDY SILT, some clay, some gravel ( TILL )

FILE NO.: **1-08-3220** LAB NO.: **1049B** 

SAMPLE DATE: March 17, 2010 SAMPLED BY: AW / JS

#### **GRAIN SIZE DISTRIBUTION**



MIT				COARSE	MEDIUM FINE		
SYSTEM		GRAVEL			SAND	SILT	CLAY
UNIFIED	COARSE	FINE	COARSE	MEDIUM	FINE		
SYSTEM		AVEL		SANI	5	SILT AND	CLAY



PROJECT: 2935 & 2955 Mississauga Road

LOCATION: Mississauga, Ontario

CLIENT: Mr. Merulla

BOREHOLE NUMBER: 1607 SAMPLE NUMBER: 2

SAMPLE DEPTH: 0.6 - 1.2 m

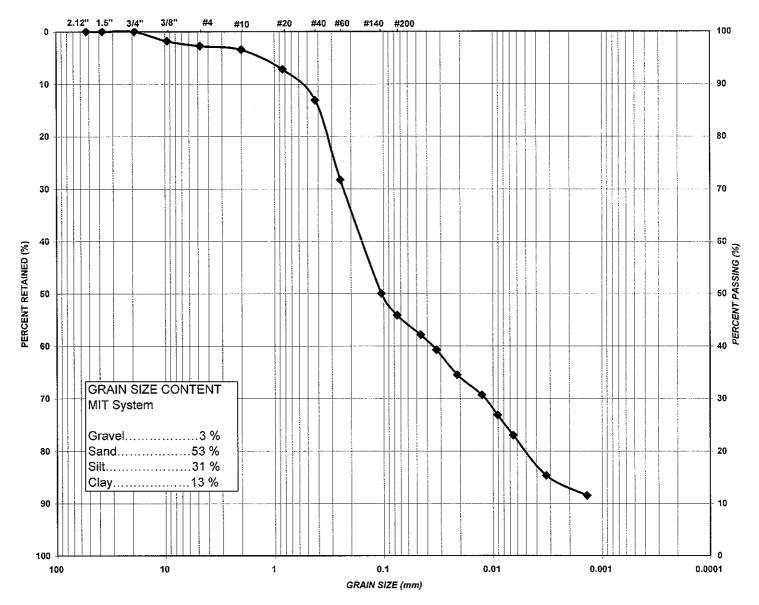
SAMPLE DESCRIPTION: SILTY SAND, some clay, trace gravel (TILL)

FILE NO.: **1-08-3220** LAB NO.: **1049C** 

SAMPLE DATE: March 17, 2010

SAMPLED BY: AW / JS

#### **GRAIN SIZE DISTRIBUTION**



MIT SYSTEM		GRAVEL			SAND	SILT	CLAY
UNIFIED SYSTEM	COARSE	FINE AVEL	COARSE	MEDIUM SAND	FINE	SILT AND	CLAY
01012	- OIN	MVLL.	<u> </u>		1		



PROJECT: 2935 & 2955 Mississauga Road

LOCATION: Mississauga, Ontario

CLIENT: Mr. Merulla

BOREHOLE NUMBER: 1608 SAMPLE NUMBER: 2

SAMPLE DEPTH: 0.6 - 1.2 m

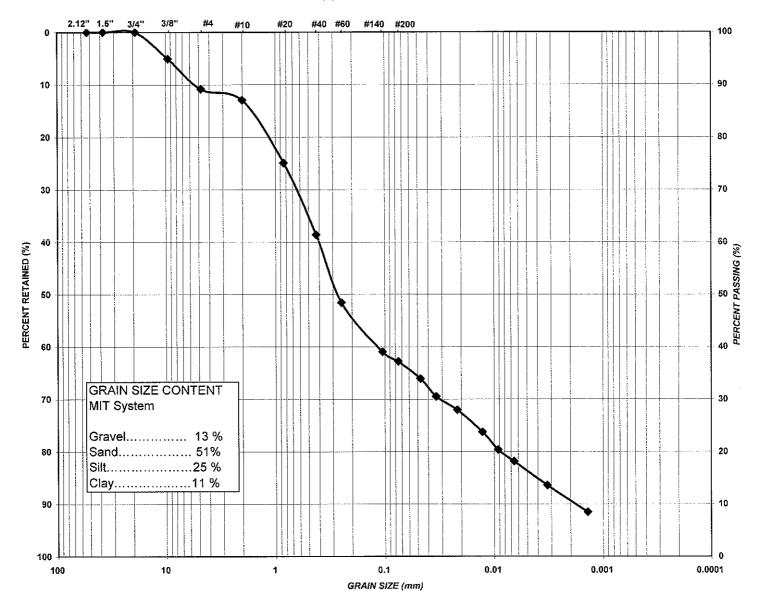
SAMPLE DESCRIPTION: SILTY SAND, some gravel, some clay ( TILL )

FILE NO.: **1-08-3220** LAB NO.: **1049D** 

SAMPLE DATE: March 17, 2010

SAMPLED BY: AW / JS

#### **GRAIN SIZE DISTRIBUTION**



MIT				COARSE	MEDIUM	FINE		
SYSTEM	;	GRAVEL			SAND		SILT	CLAY
UNIFIED	COARSE	FINE	COARSE	MEDIUM		FINE		
SYSTEM	GR	AVEL		SAN	-		SILT AND	···



PROJECT: 2935 & 2955 Mississauga Road

LOCATION: Mississauga, Ontario

CLIENT: Mr. Merulla

BOREHOLE NUMBER: 1611 SAMPLE NUMBER: 1

SAMPLE DEPTH: 0.0 - 0.6 m

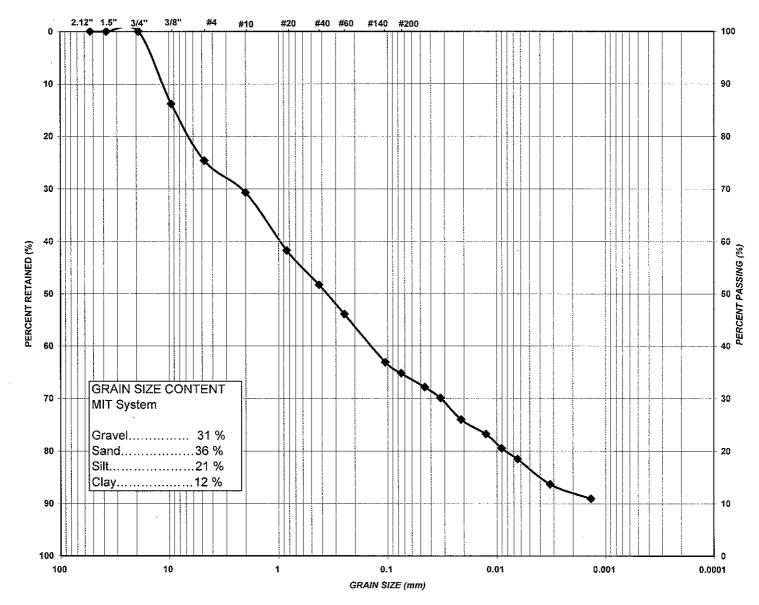
SAMPLE DESCRIPTION: GRAVELLY SAND, silty, some clay

FILE NO.: **1-08-3220** LAB NO.: **1049E** 

SAMPLE DATE: March 17, 2010

SAMPLED BY: AW / JS

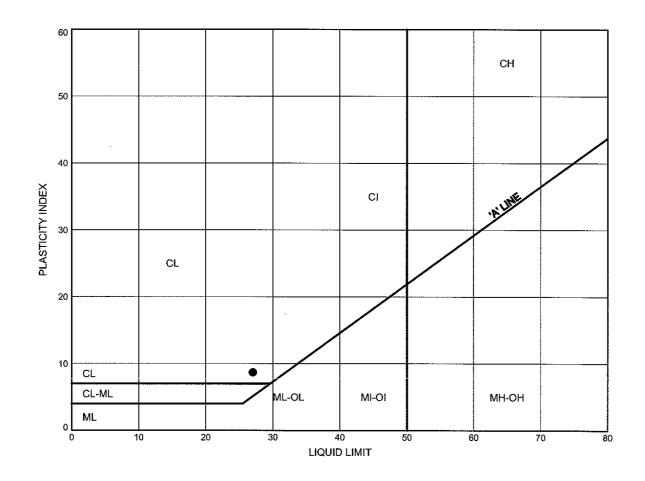
#### **GRAIN SIZE DISTRIBUTION**



MIT				COARSE	MEDIUM	FINE		
SYSTEM		GRAVEL			SAND		ŞILT	CLAY
UNIFIED	COARSE	FINE	COARSE	MEDIUM	'	FINE		
SYSTEM	GR	AVEL		SAND			SILT AND	CLAY

TERRAPROBE INC.





SYMBOL	BOREHOLE	DEPTH (m)	ELEVATION (m)
•	1603	0.9	97.2

Liguid Limit (WL) = 27.0
Plastic Limit (WP) = 18.3
Plasticity Index (IP) = 8.7
Natural Water Content (WN) = 16

Soil Classification: Slightly plastic, slight or low compressibility

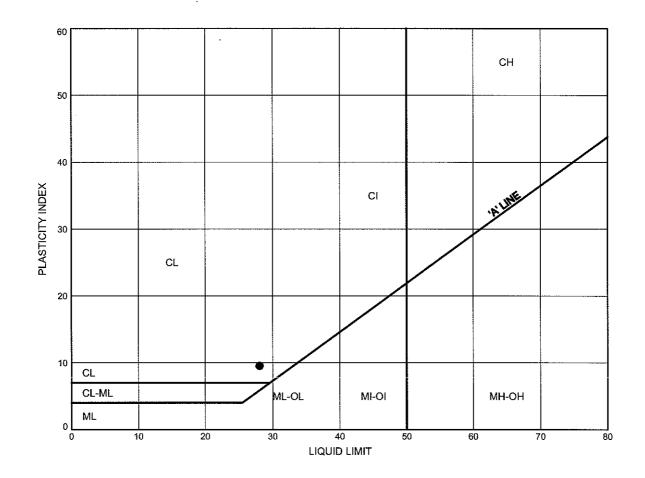
Date March 2010

Project 1-08-3220



Prep'd J.S.
Chkd. B.S.

ALTR 1-08-3220 MISSISSAUGA RD.GPJ 3/26/10



SYMBOL	BOREHOLE	DEPTH (m)	ELEVATION (m)
•	1605	0.9	97.2

Liguid Limit (WL) = 28.0
Plastic Limit (WP) = 18.5
Plasticity Index (IP) = 9.5
Natural Water Content (WN) = 16

Soil Classification: Slightly plastic, slight or low compressibility

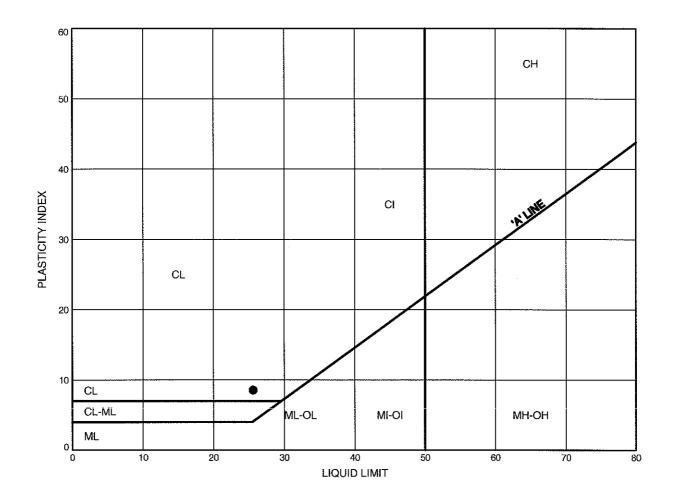
Date March 2010

Project 1-08-3220....



Prep'd J.S. Chkd B.S.

ALTR 1-08-3220 MISSISSAUGA RD.GPJ 3/26/10



SYMBOL	BOREHOLE	DEPTH (m)	ELEVATION (m)
•	1607	0.9	97.1

Liguid Limit (WL) = 25.6Plastic Limit (WP) = 17.1Plasticity Index (IP) = 8.5Natural Water Content (WN) = 16

Soil Classification: Slightly plastic, slight or low compressibility

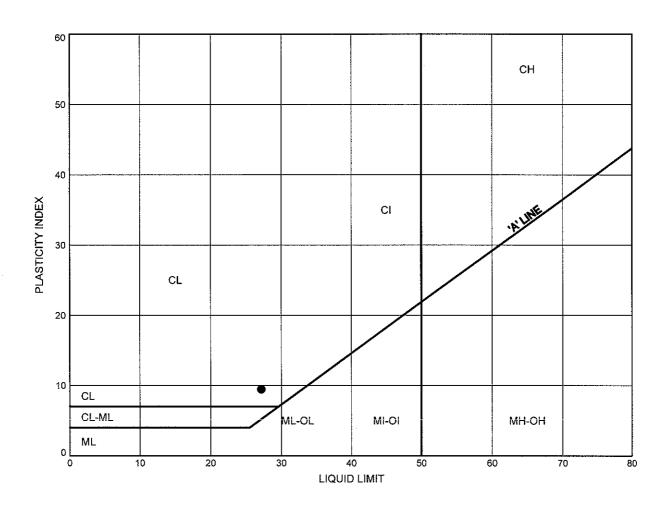
Date March 2010

Project .1-08-3220....



Prep'd J.S.
Chkd. B.S.

ALTR 1-08-3220 MISSISSAUGA RD.GPJ 3/26/10

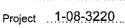


SYMBOL	BOREHOLE	DEPTH (m)	ELEVATION (m)
•	1611	0.3	97.8

Liguid Limit (WL) = 27.1
Plastic Limit (WP) = 17.7
Plasticity Index (IP) = 9.4
Natural Water Content (WN) = 13

Soil Classification: Slightly plastic, slight or low compressibility

Date March 2010

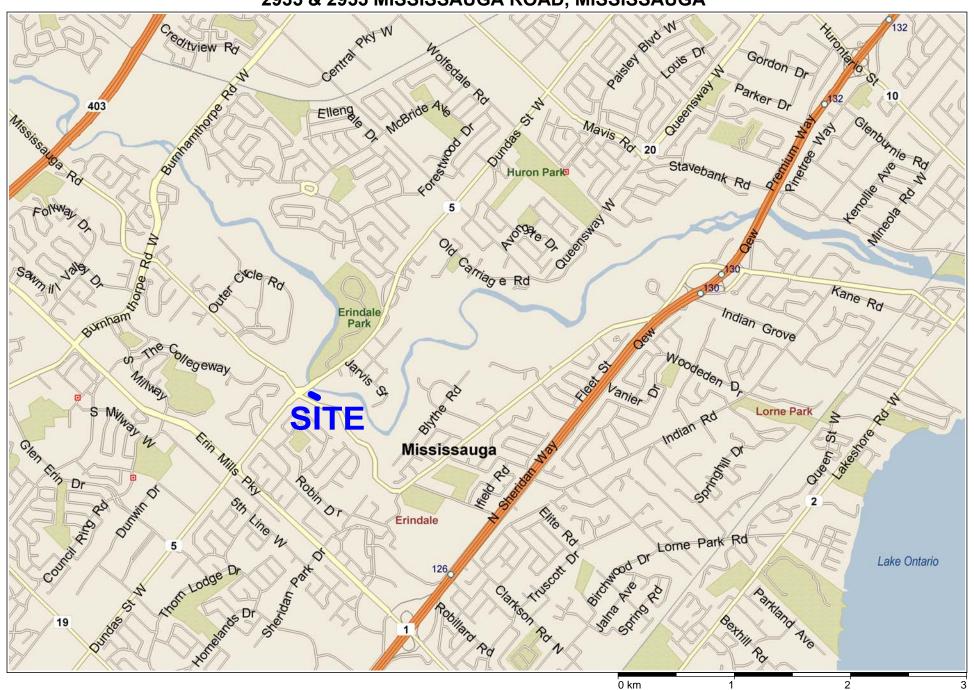




# **FIGURES**



#### 2935 & 2955 MISSISSAUGA ROAD, MISSISSAUGA



s\fileserver\Terraprobe Limited\All Projects\2008 Files\1-08-3200 to 1-08-3299\1-08-3220\A Dwgs, Logs\autoCAD\1-08-3220 FIG 3A Rev 2 MARCH 2010 - Standard

# **PHOTOGRAPHS**

TERRAPROBE INC.



# 2935 - 2955 Mississauga Road, Mississauga, Ontario



Photograph A: Looking from west side of the fence towards the site. Relatively 'recent' graded fill located on the east side of the fence.

TERRAPROBE INC. File No. 1-08-3220

# 2935 - 2955 Mississauga Road, Mississauga, Ontario



Photograph B: Another view (looking north) of the relatively 'recent' fill area, no significant vegetation cover / growth.

TERRAPROBE INC. File No. 1-08-3220

# 2935 - 2955 Mississauga Road, Mississauga, Ontario



Photograph C: A view of southern portion of the site (inside the silt fence). This area includes patchy vegetation growth, and does not appear to include 'recent' fill.

TERRAPROBE INC. File No. 1-08-3220